CRA District

EXHIBIT A - CRA MAP WITH IDEN



City of Naples, Florida Forty One Ten 1075 Central Date: 4/17/2014 Baker Park **Legend**

Mangrove Bay Naples Square **CRA District**

1,500 1,000 0 250 500

TFeet

1 inch = 1,000 feet

The City of Napkes assumes no liability for errors or omissions in the accuracy of this map. Data Sources: Perroel - Collier County Property Appraiser June 2012. Transportation Planimetrics - Collier County Property Appraiser January 2012. All Information shown is for reference only. Technology Service GIS Division 295 Riverside Circle Naples, FL 34102

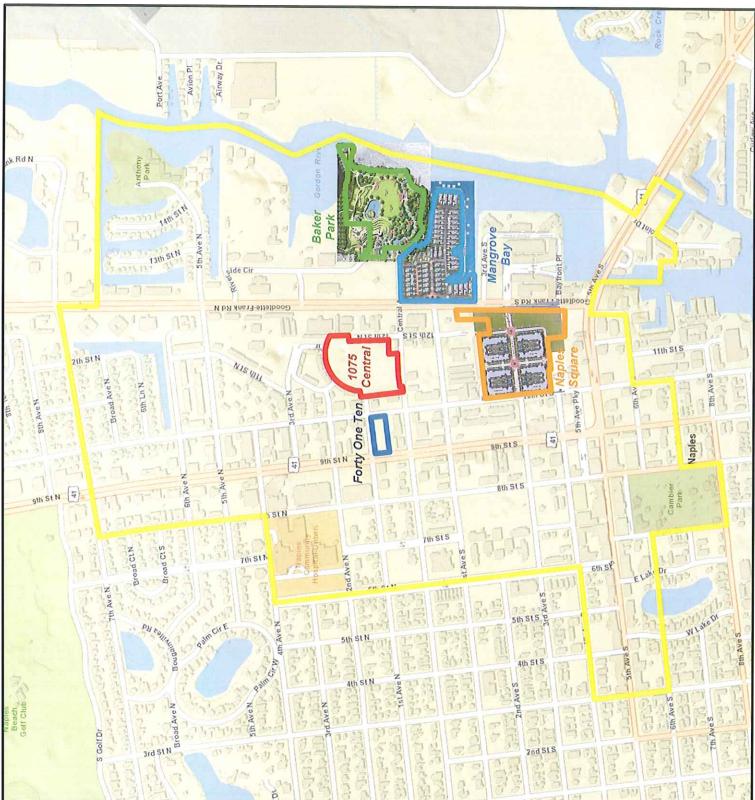


EXHIBIT B - TRAFFIC IMPACT STUDY- NAPLES DAILY NEWS SITE JMB TRANSPORTATION ENGINEERING, INC.

TRAFFIG/TRANSPORTATION ENGINEERING & PLANNING SERVICES

TRAFFIC IMPACT STATEMENT

For

1075 CENTRAL

(Central Avenue, Naples, Florida)

October 23, 2013

Prepared by:

JMB TRANSPORTATION ENGINEERING, INC. 761 21ST STREET NW NAPLES, FLORIDA 34120 (239) 919-2767 CERTIFICATE OF AUTHORIZATION NO. 27830

(PROJECT No. 130911)

JAMES M. BANKS, P.E. FLORIDA REG. NO. 43860 10-23-2013

DATE

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Conclusions

Based upon the findings of this report, it was determined that the proposed development of 1075 Central will not have a negative impact upon the surrounding road network. It was verified that all roadways, within the project's area of influence, currently have a surplus of capacity and can accommodate the traffic associated with the proposed 220 apartment units and 3,000 square feet of specialty retail use. As determined, the road network will continue to operate at acceptable levels of service for 2018 project build-out conditions and the project will not create any off-site transportation deficiencies that need to be mitigated.

Scope of Project

1075 Central is a proposed apartment complex that will consist of 220 dwelling units, and 3,000 square feet of specialty retail use. The 8.8 +/- acre site is located on the northwest corner of Central Avenue and 12th Street North, within the City of Naples. Vehicular ingress/egress to the site will be via access to Central Avenue, access to 1st Avenue North and 12th Street North. For additional site details, refer to the MCP prepared by MHK Architecture & Planning.

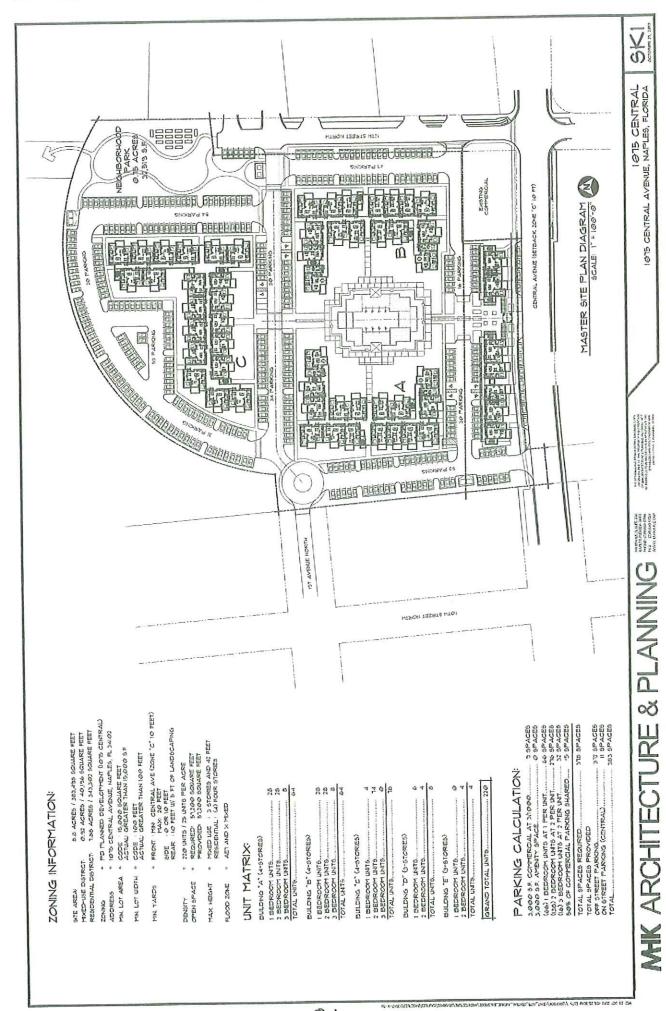
Table A
Proposed Land Use

Propose	ed Land Use
Proposed Land Use	Number of Units or Square Feet
Apartment Complex	220 Dwelling Units
Specialty Retail	3,000 Square Feet

As an alternative to the proposed land uses, the site could be developed as a commercial center consisting of 220,000 square feet of retail and office space. Table B provides a detail of the site's potential build-out.

Table B
Potential Development

Potential Development	Number of Units
Retail	70,000 Square Feet
Office	150,000 Square Feet



Project Generated Traffic

Traffic that can be expected to be generated by 1075 Central was estimated based upon the guidelines established by the Institute of Transportation Engineers, Trip Generation Manual, 9th Edition. That is, historical traffic data collected at similar land uses was relied upon in estimating the project's traffic. It was concluded that land use codes "Apartments" (LUC 220) and :Specialty Retail" (LUC 826) were most appropriate in estimating the new trips associated with the proposed project.

As determined, the project will generate 1,536 daily trips and 120 vph & 143 vph new trips during the AM and PM peak hours, respectively. Table 1A depicts the computations performed in determining the project's new trips, as well as for the alternative land uses. Table B provides a summary of the trip generation computation results that are shown in Table 1.

Table C
New Site-Generated Trips
(Summation of Table 1A)

	Daily Weekday	AM Peak Hour	PM Peak Hour
	Trips Generated	Trips Generated	Trips Generated
	(ADT)	(vph)	(vph)
LUC 220 + LUC 826	1,536	120	143

As an alternative to the proposed land uses, the site could be developed as a commercial complex consisting of 220,000 square feet of retail and office land uses. Table D provides a detail of the potential trips that would be generated by the site's potential build-out.

Table D
Potential Site-Generated Trips
(Summation of Table 1B)

	Daily Weekday	AM Peak Hour	PM Peak Hour
	Trips Generated	Trips Generated	Trips Generated
	(ADT)	(vph)	(vph)
LUC 750 + LUC 820	4,767	328	490

TABLE 1A TRIP GENERATION COMPUTATIONS 1075 CENTRAL

PROPOSED DEVELOPMENT

Land Use <u>Code</u> 220 826	Land Use Description Apartment Specialty Retail		nedule 20 Units 100 s.f.		
Land Use Code LUC 220	<u>Trip Period</u> Daily Traffic (ADT) =	Trip Generation Equation T = 6.65(X) =	Total Trips 1,463 ADT	Trips Enter	· <u>/Exit</u>
	AM Peak Hour (vph) =	T = 0.49(X)+3.73 = 20% Enter/ 80% Exit =	112 vph 139 vph	22 / 89 90 / 49	vph vph
	PM Peak Hour (vph) =	T = 0.55(X)+17.65 = 65% Enter/ 35% Exit =		30 / 10	1.15.1
******	*********	***********	********		
******	**********	***********	*********		
LUC 826	Daily Traffic (ADT) =	T=44.32(X) =	133 ADT		
	AM Peak Hour (vph) =	T=6.84(X) = 48% Enter/ 52% Exit =	21 vph	10 / 11	vph
	PM Peak Hour (vph) =	T=2.71(X) = 44% Enter/ 56% Exit =	8 vph	4 / 5	vph
Pass-by ar	d Interanl Capture Trips =	45%			
	New Daily Traffic (ADT) =	(ADT) x (% of New Trips)	73 ADT		
	New AM Peak Hour (vph) =	(AM) x (% of New Trips) 44% Enter/ 56% Exit =	8 vph	4 / 5	vph
	New PM Peak Hour (vph) =	(PM) x (% of New Trips) 48% Enter/ 52% Exit =	4 vph	2 / 2	vph
*****	*********	*********	*********	*	
Total Nev	v Daily Traffic (ADT) =		1,536 ADT		
	AM Peak Hour (vph) =		120 vph	26 /	94 vph
	PM Peak Hour (vph) =		143 vph	92 /	51 vph

TABLE 1B TRIP GENERATION COMPUTATIONS 1075 CENTRAL

POTENTIAL DEVELOPMENT

Land Use <u>Code</u> 750 820	Land Use Description Office Park Shopping Center	15	Schedule 50,000 s.f. 70,000 s.f.	
Land Use <u>Code</u> LUC 750	Trip Period Daily Traffic (ADT) =	Trip Generation Equation (Based upon S.F.) T= 11.42(X) =	Total Trips 1,713 AD	<u>Trips Enter/Exit</u> T
	AM Peak Hour (vph) =	T= 1.71(X) =	257 vp	h 228 / 28 vph
	PM Peak Hour (vph) =	89% Enter/ 11% Exit = T= 1.48(X) = 14% Enter/ 86% Exit =	222 vp	
******	*********	**********	***********	**
LUC 820	Daily Traffic (ADT) =	Ln(T) = 0.65Ln(X) + 5.83 =	5,386 A	Т
	AM Peak Hour (vph) =	Ln(T) = 0.61Ln(X)+2.24 = 62% Enter/ 38% Exit =	125 vp	h 78 / 48 vph
	PM Peak Hour (vph) =	Ln(T) = 0.67Ln(X)+3.31 = 48% Enter/ 52% Exit =	472 vp	h 226 / 245 vph
Pass-by Tr	ips per ITE= Ln(T) = 0.29Ln(X) + 5.	00 =	43% Pass-by Rate	
	New Daily Traffic (ADT) =	(ADT) x (% of New Trips)	3,054 A	DT
	New AM Peak Hour (vph) =	(AM) x (% of New Trips) 62% Enter/ 38% Exit =	71 vj	oh 44 / 27 vph
	New PM Peak Hour (vph) =	(PM) x (% of New Trips)	268 v	oh 128 / 139 vph
*****	*********	48% Enter/ 52% Exit = ************************************	*******	***
Total Nev	v Daily Traffic (ADT) =		4,767 A	DT
	AM Peak Hour (vph) =		328 A	.DT 272 / 55 vph
	PM Peak Hour (vph) =		490 A	ADT 326 / 164 vph

Existing + Committed Road Network

Table 2A provides a detail of the surrounding E + C road network. Table 2A depicts the minimum level of service performance standards and capacity for the roads within the project's are of influence.

Project Traffic Distribution

The project's traffic was distributed to the surrounding roadway network based upon logical means of ingress/egress; current and future traffic patterns in the area; location of surrounding businesses and commercial centers. Table 2A provides a detail of the traffic distributions based on a percentage basis and by volume.

Area of Significant Impact

The area of significant impact was determined based upon typical impact criteria that is commonly referred to as the 2%, 2% and 3% criteria (i.e., if the project's traffic is 2% or more of a roadway's adopted level of service capacity for the two closest roads and 3% for roads thereafter, then the project has a significant impact upon that link). Table 2A describes the project traffic distributions and the level of impact on the surrounding roadways. Roads and intersections that were identified as being within the projects area impact are shown in Table 2A.

TABLE 2A PROJECT'S AREA OF IMPACT

'roject Traffic Peak Hour 2-Way (vph) =	ur 2-Way (vph) = 143									
		Road	PK Direction Service Volume LOS Standard	LOS Service Pk Hour 2-Way Volume (vph)	ect Traffic	Project Traffic PK Direction Volume (vphpd)		Percent Impact	Significant Impact NO	
Oth Street	5th Ave S. to Central Central to 5th Ave N.	20-C 20-C	υU	1400	2%	7		0.51%	20	
entral Ave	Tamiami Trail to 10th Street 10th Street to Goodlette Frank	20-C	υυ	850 850	50% 40%	72 57	7%	8.41% 6.73%	YES	
300dlette-Frank Rd	Tamiami Trail to Central Ave Central Ave to Golden Gate Pkwy	09	шш	5800		36	2%	0.62%	0 0 0 0	
'amiami Trail	5th Ave S. to Golden Gate Pkwy East of Goodlette-Frank	09	шш	5800 5800	30%	43 29	2%	0.74%	0 0 2 2	

2013 thru 2018 Project Build-out Traffic Conditions

In order to establish 2013 thru 2018 project build-out traffic conditions, historical and current traffic count data was collected from the City of Naples. Based upon the 2002 thru 2013 traffic count report, an annual growth rate was determined and then applied to background traffic volumes in order to forecast future 2018 conditions. In those cases where the historical growth trend was found to be less than 0.5% per year, a minimum growth rate of 0.5% was applied. After the correct adjustments were computed, the project generated traffic was then added to the 2018 background traffic. Tables 2B and 2C provide a summary of the 2013 thru 2018 traffic conditions and the roadways' level of service and remaining available capacity.

As shown, all project impacted roadways will continue to operate at the City's adopted minimum level of service thresholds at project build-out.

TABLE 2B 2013 & 2018 ROADWAY LINK VOLUMES

2018 Peak Hour PK Direction Background (vphpd) 383 383	740	3363	3448 5433
2013 Total Pk Hr PK Direction Background (vphpd) 374 374	722 722	3280	3363 5299
Trip Bank (vphpd) 0	00	00	00
Min Growth Rate 0.50%	0.50%	0.50%	0.50%
Growth Rate -1.96%	-1.36%	-0.88% -0.88%	-0.44%
Years of Growth 11	7 7	7 7	# F
2013 Traffic (<u>vph)</u> 374 374	722	3280	3363 5299
2002 Traffic (vph) 465	839 839	3616 3616	3532 5684
5th Ave S. to Central Central to 5th Ave N.	Tamiami Trail to 10th Street 10th Street to Goodlette Frank	Tamiami Trail to Central Ave Central Ave to Golden Gate Pkwy	5th Ave S. to Golden Gate Pkwy East of Goodlette-Frank
0th Street	Sentral Ave	300dlette-Frank Rd	famiami Trail

TABLE 2C 2018 ROADWAY LINK VOLUME/CAPACITY ANALYSIS

	5th Ave S. to Central Central to 5th Ave N.	2013 Peak Hour 2-Way (<u>vph)</u> 374 374	2013 Peak Hour 2-Way <u>LOS</u> C	2018 Peak Hour 2-Way Background (vph) 383 383	2018 Peak Hour 2-Way Background C C	Project Peak Hour 2-Way (vph) 7	2018 Build-Out Peak Hour 2-Way (vph) 391	Service Vol. Peak Hour 2-Way (vph) 1400	2018 Build-Out Peak Hour 2-Way V/c Ratio 0.28 0.28	2018 Build-Out Peak Hour 2-Way LOS B
Tamiami Toth Stree	Tamiami Trail to 10th Street 10th Street to Goodlette Frank	722 722	υυ	740	υυ	72 57	812	850	0.95	υυ
Tamiami . Central A	Tamiami Trail to Central Ave Central Ave to Golden Gate Pkwy	3280 3280	υυ	3363	υυ	36	3399	5800	0.59	υO
5th Ave S East of G	5th Ave S. to Golden Gate Pkwy East of Goodlette-Frank	3363 5299	ОШ	3448 5433	ОШ	29	3491	5800	0.60	ОШ

APPENDIX

Support Documents

Two-way Volumes (Vehicles Per Day) For collector streets Arterials. In the City Of Naples

PEAK																																				
2013																																		A 10 10 10 10 10 10 10 10 10 10 10 10 10		
	NOOF NOOF																																			-
2013																																				
PEAK	HOUR	1,401	2,404	2,698	2,549	2,351	2,397	3,512	701	210	364	98	37.0	351	291	269	412	296	166	325	239	288	466	300	458	525	518	379	364	641	402	254	435	412	244	
2013		16,083	23,744	31,740	30,960	27,852	28,815	41,958	7,360	2,012	3,522	893	3,367	3,421	2,415	2,741	3,838	3,128	1,698	3,516	2,274	3.036	4.771	2.888	4,259	6.805	5,729	4,149	3.711	6.063	4.856	2.592	4.828	A 606	2 123	4,100
PEAK	HOUR	1,296	2,315	2,771	2,624	2,448	2,481	3,644	672	216	319	95	348	371	224	259	409	308	155	386	280	266	475	296	492	558	533	428	316	639	412	299	402	107	2000	777
2013		16,243	23,910	33,039	32,523	29,380	31,379	44,756	7,044	2,031	3,240	854	3,638	3,589	2,106	2,767	3,872	3,219	1.673	3.651	2379	2,002	7000	2002	4 658	6.657	5.848	4.335	3 707	5716	5,102	2,120	4,040	4,322	114.4	2,020
1ST QTR PEAK	HOUR	2.061	3.280	3.778	3.633	3,363	3,775	5,299	1,506	554	549	115	756	895	399	432	899	751	289	476	274	200	120	124	726	708	803	828	765	400	000	000	000	01.0	3//	513
MAR. 2013		24 634	35 042	44.600	43 848	40 279	44.522	62.116	16,119	5,744	6,692	1,194	7,862	8,750	3,797	4.177	6.034	6821	2 903	A 04A	4,000	3,020	2,030	6,650	7.740	1,110	0.225	0,000	0,00	2,044	0,240	1,135	5,553	6,6/5	8,017	4,550
ARTERIAL	COLLECTOR STREET	(See all Naviority (See 1996)	GOLDEN GATE PAWT (CK 555)	GOUDLEITE ROAD (CN 331)	US 41 (N OF CR 600)	US 41 (5 OF CR 666)	US 41 (6 AV N/ AV N)	100 41 (W O) O(03)	DABKEHOPE DRIVE	CHANGE BLAD N	LAPROLIR DRIVE	CREECH ROAD	MOORING LINE DRIVE	CDAVTON ROAD	ON TOWN AVENUE NORTH	ZZIND AVENUE	CNORTH DINAL	TENOCHMANN DEVO	GULFSHURE BLVD	BANYAN BLVD	7TH AVENUE NOKIH	10TH STREET	5TH AVENUE NORTH	CENTRAL AVENUE	8TH STREET	3RD AVENUE SOUTH	5TH AVENUE SOUTH	9TH STREE!	BROAD AVENUE SOUTH	3RD STREET	GORDON DRIVE	SANDPIPER ST	GULFSHORE BLVD SO	4TH AVENUE NORTH	NEAPOLITAN WAY	WEST RD
TRAFFIC	STATION	노 노	1	1	132	16	19	23	24	200	27	20	000	000	0.4	5	44	45	48	49	55	56	57	62	63	64	70	72	92	22	79	83	82	86	88	16

QUARTERLY TRAFFIC COUNTS

	50 5520	COMMO	T	T	T	Γ	T	T	T	Γ	T	Т	T	1	T	Γ	T	T	Ī	T	T	1	T	Т	T	Г		Ι	T	T	T	1	Г	T	Т	Т
2002	PEAK	1.995	3.616	4,440	3,775	3,532	3,750	5,684	1,592	674	651	123	226	1.119	356	435	688	944	406	551	465	418	839	584	856	824	839	617	526	1.033	518	713	653	773	644	
MAXIMUM	2002	24.078	42,080	55,990	48,646	45,854	46,682	89,058	18,297	6,939	6,452	1,334	10,554	11,464	3,474	4,495	6,659	10.230	4.106	5,889	4,356	4,629	8,407	5,587	8,070	10,503	9,554	5,957	5,954	9,745	6,506	6,116	7,031	8,355	6,239	
4TH QTR	PEAK HOUR	1,966	3,091	4,033	3,400	3,016	3,161	4,912	1,085	354	373	128	607	675	374	393	714	237	285	467	425	412	640	428	645	719	691	540	428	879	488	404	498	708	501	
DEC.	2002	22,959	35,380	48,371	41,365	36,518	38,127	59,364	11,950	3,753	4,271	1,287	5,668	6,785	2,897	4,056	7,127	2,338	2,711	4,850	4,004	4,307	6,414	4,288	6,453	8,822	7,432	5,478	4,272	8,394	5,662	4,140	5,687	7,008	4,646	
3RD QTR	HOUR	1,473	2,639	3,323	2,831	2,620	2,753	4,150	777	215	414	105	379	442	296	284	530	376	204	417	383	349	260	323	450	636	547	396	347	628	412	296	460	469	346	
	7007	17,733	28,683	42,211	34,507	31,701	31,515	49,098	7,930	2,177	3,780	1,006	3,951	4,334	2,423	3,031	4,877	3,907	1,922	4,263	3,489	3,743	5,747	3,220	4,999	7,729	6,364	4,261	4,027	6,873	5,126	3,090	4,994	4,791	2,893	
JUN. ZND QTR	HOUR	1,557	3,116	3,557	3,017	2,797	2,656	4,271	812	218	372	92	426	466	207	279	585	407	204	439	402	410	596	407	486	679	620	428	392	292	454	317	523	457	305	
JUN.	7007	18,521	35,072	42,024	36,463	33,626	32,882	51,859	9,071	2,452	4,031	884	4,732	4,974	2,267	3,097	5,360	4,617	2,204	4,704	3,430	4,274	6,021	4,034	5,229	8,073	6,811	4,538	4,410	7,241	5,157	3,593	5,348	5,115	3,006	
1ST QTR	HOUR	1,995	3,616	4,440	3,775	3,532	3,750	5,684	1,592	674	651	123	977	1,119	356	435	688	944	406	551	465	418	839	584	856	824	839	617	526	1,033	518	713	653	773	644	
MAR.	7007	24,078	42,080	55,990	48,646	45,854	46,682	69,058	18,297	6,939	6,452	1,334	10,554	11,464	3,474	4,495	6,659	10,230	4,106	5,889	4,356	4,629	8,407	5,587	8,070	10,503	9,554	5,957	5,954	9,745	905'9	6,116	7,031	8,355	6,239	
ARTERIAL	COLLECTOR STREET	GOLDEN GATE PKWY (CR 886)	GOODLETTE ROAD (CR 851)	US 41 (N OF CR 886)	US 41 (S OF CR 886)	US 41 (6 AV N/7 AV N)	US 41 (W OF CR 851)	US 41 (E OF CR 851)	PARKSHORE DRIVE	GULFSHORE BLVD	HARBOUR DRIVE	CREECH ROAD	MOORING LINE DRIVE	CRAYTON ROAD	22ND AVENUE NORTH	ORCHID DRIVE	FLEISCHMANN BLVD	GULFSHORE BLVD	BANYAN BLVD	7TH AVENUE NORTH	10TH STREET	5TH AVENUE NORTH	CENTRAL AVENUE	8TH STREET	3KD AVENUE SOUTH	SIH AVENUE SOUTH	9TH STREET	BROAD AVENUE SOUTH	3RD STREET	GORDON DRIVE	SANDPIPER ST	GULFSHORE BLVD SO	4TH AVENUE NORTH	NEAPOLITAN WAY	WEST RD	
TRAFFIC	STATION	60	10	15	9	19	23		30										1			57	1	T					1		1	82			91	

T	ABLE 1				Urba	nizea A	reas			1	2/18/12
	INTERRU	PHED EV	D)/// EA/CII	ITIES			UNINTER	RUPTED FL	OW FACILI	TIES	
	STATE SIG							FREEWA	AYS		
								Core Urba	nized		Б
	Class I (40 mp	on or nigne B	C C	D D	E	Lanes	В	C	D	0	E 84,600
anes	Median Undivided	В	16,800	17,700	**	4	47,400	64,000	77,90 116,60		30,600
2	Divided	*	37,900	39,800	**	6	69,900	95,200	154,30	175 S	76,600
4	Divided	*	58,400	59,900	专幸	8	92,500	126,400			22,700
6 8	Divided	*	78,800	80,100	**	10	115,100	159,700			268,900
				anged limit	,	12	162,400	216,700	230,00	70 2	200,700
	Class II (35 m	ph or slow B	er posted C	D D	E			Urbani			Е
anes	Median	р *	7,300	14,800	15,600	Lanes	В	C	D		79,900
2	Undivided	*	14,500	32,400	33,800	4	45,800	61,500			123,300
4	Divided	*	23,300	50,000	50,900	6	68,100	93,000			166,800
6	Divided Divided	*	32,000	67,300	68,100	8	91,500	123,500			210,300
8	Divided		52,000	1 to		10	114,800	156,000	187,10	JU .	210,500
							F	reeway Adj	ustments		
	Non-State Sig	gnalized F	Roadway A	Adjustmen mes	ts		Auxiliary Lan	es		Ramp	
	(Alter	corresponding the indicat	ng state volu ed percent.)	III.O		Prese	ent in Both Dir	ections	N	letering	
	Non-State	Signalized l	Roadways	- 10%			+ 20,000			+ 5%	
			ane Adju	stments			NINTERE	IIPTED E	LOW HIG	SHWA	YS
	Median	Exclusive	Exch	isive A	ljustment		Median	B	C	D	E
Lanes	Median	Left Lane:			Factors	Lanes	Undivided	8,600	17,000	24,200	33,300
2	Divided	Yes	N		+5%	2 4	Divided	36,700	51,800	65,600	72,600
2	Undivided	No	N		-20%	6	Divided	55,000		98,300	108,800
Multi	Undivided	Yes	N		-5% -25%	U	Divided	***	5		
Multi	Undivided	No	N Y	es	+ 5%		Uninterrup	ted Flow H	ighway Ad	justmen	its
=	_	_	•			Lanes	Median	Exclusive	left lanes	Adjusur	iem factors
	One-	Way Facil	lity Adjus	tment		2	Divided	Ye			-5%
	Multiply	the correspo	onding two-c	lirectional		Multi	Undivided	Y			-5% 25%
	V	olumes in th	is table by 0),6		Multi	Undivided	N			
di	Multiply motorize rectional roadway Paved nulder/Bicycle	d vehicle vol lanes to dete	E MODE tumes shown ermine two-v nmes.)	below by nun	nber of service	does not applicate more sp not be u Calculate the Trans	shown are present and are for the auto t constitute a stand ions. The compute ecific planning ap used for corridor or tions are based on asit Capacity and G	ard and should be ard and should be ar models from w plications. The ta intersection desi planning applica Quality of Service	e used only for go hich this table is ble and deriving gn, where more to tions of the High Manual.	eueral plans derived sho computer n refined tech way Capac	ning ould be used f nodels should niques exist. ity Manual an
	ne Coverage	В	C	D	Е	² Level	of service for the	bicycle and pede	trian modes in th	is table is b	ased on num
La	0-49%	*	2,900	7,600	19,700	of moto	orized vehicles, no	t number of bicyc	ists of pedestria	iis man Pine	
	50-84%	2,100	6,700	19,700	>19,700	3 Buses	per hour shown are	only for the peak	hour in the single o	direction of t	the higher traff
	85-100%	9,300	19,700	>19,700	ন ক	flow.					
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d	(Multiply motorize directional roadway	ed vehicle vo y lanes to de	dumes show	n below by nu	II SCEVICE	volume	applicable for tha es greater than lev eached. For the bic able because there	el of service D be	come r occause	ter grade (i	neluding F) is
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	0-49%	*	*	2,800							
	50-84%	*	1,600								
	85-100%	3,800									
	BUS Me	ODE (Sch ses in peak h	eduled Fi tour in peak o	xed Route		Source	e: la Department of T	ransportation			
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& Turn Lar	ie Aujustment	.5	9
Exclusive	Exclusive	Adjustment	L
Left Lanes	Right Lanes	Factors	L
Yes	No	+5%	
No	No	-20%	
Yes	No	-5%	100
No	No	-25%	
200000	Ves	+ 5%	

One-Way Facility Adjustment

Multiply the corresponding two-directional volumes in this table by 0.6

BICYCLE MODE²

(Multiply motorized vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service volumes.)

Paved Shoulder/Bicycle

Divided Undivided

Undivided

Undivided

2

2

Multi

Multi

Lane Coverage	В	C	D	E
0-49%	坤	260	680	1,770
50-84%	190	600	1,770	>1,770
85-100%	830	1,770	>1,770	**

PEDESTRIAN MODE²

(Multiply motorized vehicle volumes shown below by number of directional roadway lanes to determine two-way maximum service volumes.)

Sidewalk Coverage	В	C	D	E
0-49%	*	*	250	850
50-84%	*	150	780	1,420
85-100%	340	960	1,560	>1,770

BUS MODE (Scheduled Fixed Route)3 (Buses in peak hour in peak direction)

E D Sidewalk Coverage В C ≥ 3 ≥ 2 0-84% > 5 ≥ 4 ≥ 2 85-100% ≥ 1

		The Residence of the Paris			
		FREEWAY	'S		
Lanes	В	C	D	Е	
4	4,120	5,540	6,700	7,190	
6	6,130	8,370	10,060	11,100	
-		11,100	13,390	15,010	
		The state of the s	16,840	18,930	
12	14,450	18,880	22,030	22,860	
8 10	8,230 10,330	11,100 14,040	13,390 16,840	18,930	

LININTERRUPTED FLOW FACILITIES

Freeway Adjustments

Auxiliary Lanes	Ramp
Present in Both Directions	Metering
+ 1,800	+ 5%

UNINTERRUPTED FLOW HIGHWAYS

Lanes	Median	В	C	D	E
2	Undivided	770	1,530	2,170	2,990
4	Divided	3,300	4,660	5,900	6,530
6	Divided	4,950	6,990	8,840	9,790

Uninterrupted Flow Highway Adjustments

Lanes	Median	Exclusive left lanes	Adjustment factors
2	Divided	Yes	+5%
Multi	Undivided	Yes	-5%
Multi	Undivided	No	-25%

Values shown are presented as peak hour two-way volumes for levels of service and are for the automobile/truck modes unless specifically stated. This table does not constitute a standard and should be used only for general planning applications. The computer models from which this table is derived should be used for more specific planning applications. The table and deriving computer models should not be used for corridor or intersection design, where more refined techniques exist. Calculations are based on planning applications of the Highway Capacity Manual and the Transit Capacity and Quality of Service Manual.

² Level of service for the bicycle and pedestrian modes in this table is based on number of motorized vehicles, not number of bicyclists or pedestrians using the facility.

³ Buses per hour shown are only for the peak hour in the single direction of the higher traffic

Cannot be achieved using table input value defaults.

** Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached. For the bicycle mode, the level of service letter grade (including F) is not achievable because there is no maximum vehicle volume threshold using table input value defaults.

Florida Department of Transportation Systems Planning Office www.dot.state.filus.planume.assterre.sus los delamit sinta

12/18/12

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	11871330	Indeed area	VIII SA	TIES			UNINTERB	UPTED FL	oW/F/ACIL	TIES	
		IPTED FLO				No. 150 per officer		FREEWA			
	STATE SIG	GNALIZE	D ARTE	RIALS		Longs	В	C	D		E
	Class I (40 r	anh ar higher	nosted spec	ed limit)		Lanes 2	2,260	3,020	3,66	0	3,940
		B	C	D	E	3	3,360	4,580	5,50	0	6,080
anes	Median	*	830	880	**	4	4,500	6,080	7,32	0	8,220
1	Undivided	*	1,910	2,000	**		5,660	7,680	9,22		10,360
2	Divided	*	2,940	3,020	**	5	7,900	10,320	12,06		12,500
3	Divided	*	3,970	4,040	**	6	7,900	10,520	10,5	7.	
4	Divided			(20)	5		Ev	eeway Adju	stments		
	Class II (35	mph or slowe	er posted spe	eed limit)	r		Auxiliary	eeway Auji		Ramp	
Lanes	Median	В	С	D	E		Lane			etering	
1	Undivided	*	370	750	800		+ 1,000			+ 5%	
2	Divided	*	730	1,630	1,700						
3	Divided	*	1,170	2,520	2,560						
4	Divided	*	1,610	3,390	3,420						
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-	Madian	Left Lanes	Right La		actors	Lanes	Median	420	840	1,190	1,640
Lanes	Median Divided	Yes	No		+5%	1	Undivided		2,560	3,240	3,590
1	Undivided	No	No		-20%	2	Divided	1,810	3,840	4,860	5,380
Multi	Undivided	Yes	No		-5%	3	Divided	2,720	3,040	4,000	5,500
Multi	Undivided	No	No		-25%					atmon	te
-	-	-	Yes		+ 5%		Uninterrup	ed Flow H	ghway Au	Adiustma	ent factors
						Lanes	Median	Exclusive			5%
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	Multip	ly the correspond	onding direct	tional		Multi	Undivided	Ye			5%
	V	olumes in this	table by 1.2			Multi	Undivided	No)	-1	<i>570</i>
dir	Multiply motorize rectional roadway	lanes to deten	mes shown be mine two-way	low by num	ber of service	are for t constitu compute plannin	shown are presente the automobile/truck te a standard and she er models from white g applications. The or intersection des n planning application	ould be used only the this table is de- table and derivin	y for general pla rived should be g computer mod refined technique	unning applic used for more els should notes exist. Cal	ations. The re specific of be used for culations are
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	0-49%	110		1,000	>1,000	of moto	of service for the bi orized vehicles, not	number of bicyc	ists or pedestria	ns using the	facility.
	50-84% 85-100%	470		>1,000	米水		per hour shown are o				
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d	neemonal toda na	volui	mes.)	esp. (
e:	dewalk Cover	age B	С	D	E		es greater than level eached. For the bicy able because there i				
310	0-49%	age B	*	140	480		able because mere i lefaults.	3 IIO maximum Y			
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		DE (Sche		ed Route	e) ³						
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Si	0-84%	> :	$5 \geq 4$ $4 \geq 3$	≥ 2	≥ 1	11/1/17	dot shite It ds pient	(III) - A S (Call) - Call			

EXHIBIT C - TRAFFIC IMPACT STUDY LIGHTHOUSE POINT



Traffic Impact Statement

LightHouse Point

Collier County, FL 2/26/2013

Prepared for:

Signature Development

Group of Naples, LLC

525 Via Veneto

Naples, FL 34108

Phone: (239) 822-7038

Prepared by:

Trebilcock Consulting Solutions, PA

1205 Piper Boulevard, Suite 202

Naples, FL 34110

Phone: (239) 566-9551

Email: ntrebilcock@trebilcock.biz

Statement of Certification

I certify that this Traffic Analysis has been prepared by me or under my immediate supervision and that I have experience and training in the field of Traffic and Transportation Engineering.

Norman J. Trebilcock, AICP, P.E.

FL Registration No. 47116

Trebilcock Consulting Solutions, PA

1205 Piper Boulevard, Suite 202

Naples, FL 34110

Company Cert. of Auth. No. 27796

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Access Management Considerations	6 - 7
Trip Generation	8
Roadway Capacity	9 -10
Access/Intersection Analysis	11 -13
Improvement Analysis / Conclusions	14

Appendix A: Project Master Site Plan (1 Sheet)

Appendix B: Trip Generation Calculations (1 Sheet)

Appendix C: Project Trip Distribution Map by Percentage (1 Sheet)

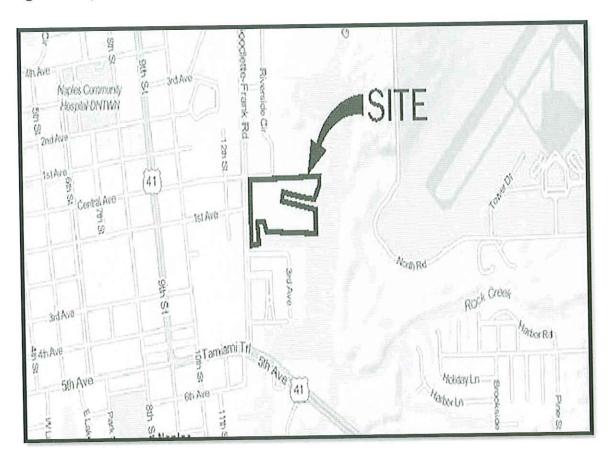
Appendix D: Scenario A - Turning Movements Detail (1 Sheet)

Project Description

The LightHouse Point project is located in City of Naples, at the southeast corner of the intersection of Goodlette-Frank Road South and Central Avenue, approximately 0.4 miles north of the intersection of Goodlette-Frank Road South and US 41. The project is located in Section 03, Township 50S, Range 25E, Collier County, FL.

The internal site circulation system proposes internal roads to connect to Goodlette-Frank Road South and Central Avenue. Refer to Fig. 1 - Project Location Map, which follows and Appendix A – Project Master Site Plan.

Fig. 1—Project Location Map



The proposed project will be developed as a residential use as follows: Scenario A – Maximum Development Potential and Scenario B – Current Proposal. Scenario A identifies the maximum potential impact that could occur under the Planned Development Rezone at build out conditions and consists of multi – family residential – 230 dwelling units. Scenario B proposes two (2) phases as follows: Phase 1 - Southern Parcel - Fee Simple Townhome - Single Family Detached Housing – 56 dwelling units, and Phase 2 - Northern Parcel (maximum phase 2 potential impact that could occur under the Planned Development Rezone) - Multi – Family Residential – 80 dwelling units. The proposed "Town Center" is considered passive incidental to residential and it is not included in the trip generation analysis.

The Traffic Impact Statement (TIS) analysis provides highest impact scenario with respect to the project's proposed trip generation at build out conditions. Based on this criterion, **Scenario A – Maximum Development Potential** represents the most intense scenario for traffic impacts.

The proposed development program is illustrated in the following tables.

Table 1A — Scenario A – Maximum Development Potential

Land Use	ITE LUC	Size*	Build-Out Year
Build Out			
Multi-Family Residential	230	230	2016

Note*: dwelling units.

Table 1B — Scenario B – Current Proposal

Land Use	ITE LUC	Total Size*	Build-Out Year
Phase 1			
S.F. Detached Housing	210	56	2015
Phase 2			
Multi-Family Residential	230	80	2016
Build Out - Total		136	

Note*: dwelling units.

Access Management Considerations

Access to the proposed project is provided by a main entrance on Goodlette-Frank Road South, directly to the east of the intersection of 1st Avenue South and Goodlette-Frank Road South. A second access (restricted to residents and emergency vehicles only) is provided on Central Avenue, east of its intersection with Goodlette-Frank Road South.

Goodlette-Frank Road is a six-lane arterial roadway and it is under jurisdiction of Collier County. Existing conditions reveal a full median opening which provides directional left turn lanes in both northbound and southbound directions and unrestricted crossing movements. Full movements are permitted without restrictions. Gaps to accommodate turning and crossing movements are provided by signalization at Central Avenue and the entrance at the Bayfront Place.

For the purpose of this analysis, access classification and standards are provided per Collier County Resolution No. 01-247 requirements. Goodlette-Frank Road South is a class 5 facility, with minimum spacing criteria as follows: a) minimum connection spacing – 330 ft; b) minimum median opening – directional (440 ft) and full (1,320 ft); c) minimum signal spacing – 2,640 ft (0.5 mile).

Access – Goodlette-Frank Road South - a summary of Collier County access classification and standards versus existing conditions is illustrated in Table 2 – Access Management Criteria on the next page. Please note Goodlette-Frank Road South is an access class 5 facility, with median restrictions and posted speed limit of 45 mph.

Table 2 — Access Management Criteria

ACCESS – GOODLETTE-FRANK ROAD SOUTH	CONNECTION SPACING (sf)	MEDIAN OPENING	
		DIRECTIONAL (ft)	FULL (ft)
Collier County Access Management Criteria – Minimum Requirements	330	440	1,320
Existing Conditions*	320	N/A	370

Note*: Approximate values – length measurements taken from Google Earth aerial photography.

The full median opening across from main entrance is recommended to be replaced by a directional median opening – with existing left turn lanes from both directions – southbound and northbound.

This modification is recommended to reduce conflicts and retain reasonable access operations.

The prohibition of direct left-turns out from the proposed project entrance, as a result of the directional median opening, may transfer the displaced left-turns to Central Avenue and Goodlette-Frank Road South traffic-signal-controlled intersection; as well as, the prohibition of direct left-turns from 1st Avenue South may transfer the displaced left-turns to U-turn lane at 3rd Avenue South entrance or at the entrance to Mariners Cove.

Per above recommendation, the remainder of this operational analysis is based on the assumption that the full median opening on Goodlette-Frank Road South is converted into a directional median opening.

Trip Generation

The project's site trip generation is based on the Institute of Transportation Engineers (ITE) <u>Trip Generation Manual</u>, 9^{th} Edition.

The residential project trip generation is illustrated in **Table 3 – Project Trip Generation**. **Scenario A** reveals the most intensive external PM values. No reductions for internal capture or pass-by trips have been taken. More details of the trip generation calculations can be found in **Appendix B – Trip Generation Calculations**. The software program Otiss – Online Traffic Impact Study Software (Version 2.1.0.15) is used to create the raw unadjusted trip generation for the project. The ITE equations – best fit, were used to generate project trips.

Table 3 — Project Trip Generation—Average Weekday*

Scenario A Maximum Development Potential		24 Hour Two-Way Volume	AM Pk Hour		PM Pk Hour	
Land Use	Size		Enter	Exit	Enter	Exit
Build-Out - Multi-Family Residential	230 du	1,328	17	84	79	40

Scenario B Current Proposal		24 Hour Two-Way Volume	AM Pk Hour		PM Pk Hour	
Land Use	Size		Enter	Exit	Enter	Exit
Phase 1 - S.F. Detached Housing	56 du	616	12	37	39	23
Phase 2 - Multi-Family Residential	80 du	530	7	36	33	17
Build Out - Total External	136 du	1146	19	73	72	40

Note*: Refer to Table 4 for the addition of this data impact on roadway network.

Roadway Capacity

The residential project trip generation is illustrated in **Table 3 – Project Trip Generation** – page 8 of this TIS.

The traffic generated by the development (most intense traffic impact - Scenario A – Maximum Development Potential) was assigned to the adjacent roadways using the knowledge of the area. The site-generated trip distribution is shown in Table 4—Roadway Capacity (Peak Hour – Two Way) and it is graphically depicted on the next page - Fig. 2—Project Distribution By Trips. The project trip distribution by percentage can be found in Appendix C.

The project impacts to the adjacent roadway network are not adverse to their level of service as each analyzed link (Goodlette-Frank Road South, Central Avenue, US 41) has sufficient capacity to accommodate project trips.

Table 4—Roadway Capacity (Peak Hour – Two Way)

	2011 Peak Hour Capacity*	2011 Peak Hour Volume*	2011 Peak Hour Surplus	Project Trips Added**	Remaining Surplus
Goodlette -Frank Rd.	5,680	2,713	2,967	48	2,919
Central Avenue	1,960	693	1,267	23	1,244
US 41 – west of Goodlette	5,420	3,732	1,688	24	1,664
US 41 – east of Goodlette	6,300	5,480	820	24	796

Note*: From City of Naples – Concurrency System Management Element – Roadways – Level of Service for 2011. Note**: Total 119 trips in the PM peak hour.



Fig. 2—Project Distribution by Trips

Access/Intersection Analysis

Central Avenue/Project Access

The proposed project has one restricted access on Central Avenue (residents and emergency vehicles only). Separate right, or left turn lanes off of Central Avenue are not proposed for this secondary access point.

Goodlette-Frank Road South/Project Access

The proposed project has its main access on Goodlette-Frank Road South, located straight across from 1st Avenue South approach. There are no operating traffic control signals at the project entrance and this location. This intersection is too close to Central Avenue for a signal.

Per the previously mentioned access management recommendation, our analysis is based on the assumption that the median opening on Goodlette-Frank Road South is a directional median opening and provides for two opposing left turn in or U-turn movements (median "pork chop").

Project main entrance was evaluated for turn lane warrants based on appropriate Collier County criteria (40 vph for right turn lane/20 vph for left turn lane). The exiting volume will be right out only, so a single egress lane is needed.

However, per "Construction Standards Handbook for Work within the Public Right-of-Way, Collier County, Florida", right turn lanes shall always be provided for existing multi-lane divided roadways (sec. III-1.b.2). Based on the facts stated above, we recommend a 210ft right turn lane length (1 vehicle queue) for the project's main driveway (northbound Goodlette-Frank Road South).

Goodlette-Frank Road South is a six-lane arterial roadway and has a posted speed limit of 45 mph. Based on FDOT Index 301, the minimum turn lane length is 185 ft (which includes a 50 ft taper) plus required queue. As a free flow right turn lane a single vehicle queue of 25 ft is recommended. The total NB right turn lane length is 210 ft

Central Avenue/Goodlette-Frank Road Intersection

This existing signalized intersection has the following approach lane conditions:

Northbound Goodlette-Frank Road approach has three (3) through lanes and one left-turn lane. Southbound Goodlette-Frank Road approach has one channelized right-turn lane (painted island), three (3) through lanes and one left-turn lane. Eastbound Central Avenue approach provides one left-turn lane and one through/right-turn lane. Westbound Central Avenue approach provides one left-turn/through/right-turn lane.

Based on this analysis, the overall project trips generated are not projected to warrant a right-turn lane, nor a left-turn lane at the westbound Central Avenue intersection approach.

The City of Naples Staff Report has identified overall off-site related Access-External/External Impacts illustrated as follows:

"1. Intersection of Goodlette-Frank Road and 1st Avenue Alignment: Currently, the existing median includes turn lanes and provides for access in all directions. The Goodlette access with the adjacent roundabout provides for acceptable throat length and capacity. Of concern long term will be growing peak hour traffic on Goodlette and the operation of the existing median opening. Should safety issues and/or intersection capacity issues arise at the median opening in the future, the potential solution of such issues will be median modification to provide directional controls or median closure. Should direct access via southbound Goodlette be constrained in the future, the secondary access via Central Avenue would become the primary point of access. Permitting of access via Goodlette shall be subject to Collier County policies and controls; it is anticipated that a northbound right turn lane will be both needed and required to maintain capacity on Goodlette, subject to intersection improvements at Central Avenue.

2. Intersection of Goodlette-Frank Road and Central Avenue and Driveway Access via Central Avenue: The proposed plat includes a provision to have a second development access via Central Avenue. In the distant future based on increasing levels of traffic on Goodlette-Frank, potential median controls and the safety of using a signalized intersection, southbound development trips are anticipated to shift from the main entrance to the Central Avenue entrance. Should this scenario occur, future improvements need to be identified for the intersection of Goodlette and Central Avenue. These improvements potentially include a northbound right turn lane and an added westbound lane. The cost of these future improvements should be shared between the City and the new development. The sharing of cost can be via right-of-way commitments and/or a combination of right-of-way commitments and construction cost based on generated traffic volumes."

The LightHouse Point project proposes to provide an easement for a northbound right-turn lane on Goodlette-Frank Road South to fulfill its "fair share" commitment to intersection improvements (should the value of this easement exceed the project's "fair share", offsetting impact fee or other credits may be available to the project). The proposed turn lane length is estimated at 235 ft, which would accommodate a 50 ft taper, deceleration and queue. This is based on a 45 mph design speed. In addition a minimum 30 ft interior radius return is recommended.

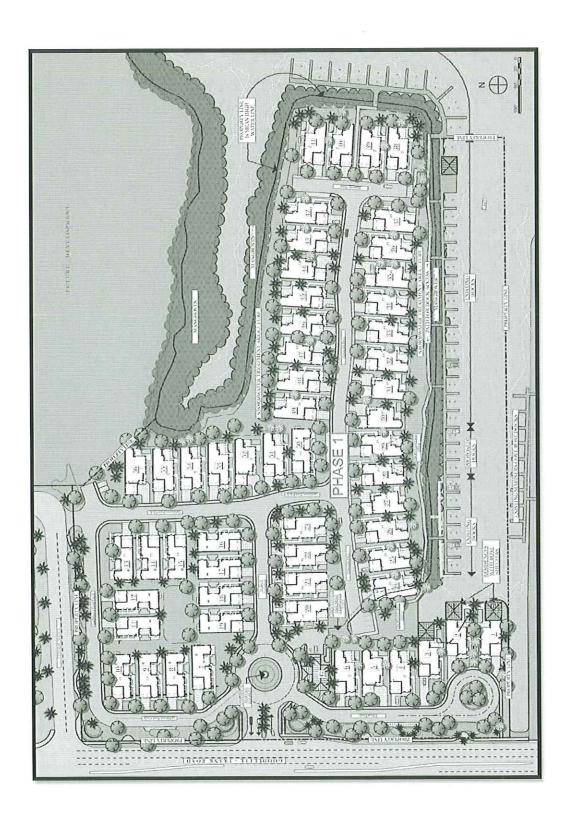
Improvement Analysis / Conclusions

Based on the results of this analysis, there is adequate and sufficient roadway capacity to accommodate the proposed development generated trips without adversely affecting adjacent roadway network level of service. However, operational offsite improvements are recommended as follows:

- 1. Modifications to the median opening on Goodlette-Frank Road South at the property main entrance convert the full median to a directional opening "pork chop" configuration northbound/southbound to accommodate for two opposing left turn or U-turn movements.
- 2. Goodlette-Frank Road South Main Entrance northbound deceleration lane right turn 210 ft lane (includes 50 ft taper) plus a 30 ft interior radius return.
- 3. Goodlette-Frank Road South and Central Avenue Intersection westbound approach right turn lane / left turn lane, and northbound approach right turn lane, "fair share" contribution due to project stacking and queuing. An easement for a northbound right-turn lane on Goodlette-Frank Road South at the intersection with Central Avenue will fulfill the land owner's "fair share" commitment (should the value of this easement exceed the project's "fair share", offsetting impact fee or other credits may be available to the project). The proposed turn lane length is estimated at 235 ft, which would accommodate a 50 ft taper, deceleration and queue. In addition, a 30 ft interior radius return is recommended.

Appendix A: Project Master Site Plan (1 Sheet)

Light House Point – Traffic Impact Statement - February 2013



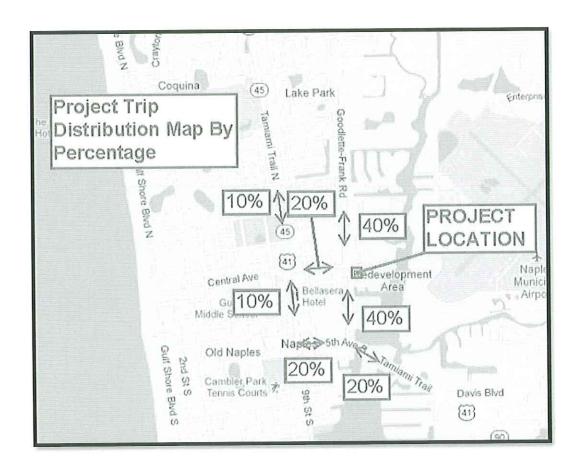
Appendix B: Trip Generation Calculations (1 Sheet)

Date: 2	/18/2013	int - Scenario A	No: City:		Naples			
State/Province:			Zip/F	ostal Code:				
Country:	Collier County		Clier	it Name:				
Analyst's Name:			Editi	on:	9th	9th		
Land Use	Size	Daily - 2 Wa	y Volum e	Volume PM Peal		AM Peak Hour		
Land Osc	Size	Entry	Exit	Entry	Exit	Entry	Exit	
230 - Residential Condominium/Townhous	e 230 ⁽⁹⁾	664	664	79	40	17	84	
Reduction		0	0	0	0	0	0	
Internal	-	0	0	0	0	0	0	
Pass-by		0	0	0	0	0	0	
Non-pass-by		664	664	79	40	17	84	
Total		664	664	79	40	17	84	
Total Reduction		0	Ö	0	0	0	0	
Total Internal		0	0	0	0	0	Ô	
Total Pass-by		0	0	0	0	0	0	
Total Non-pass-by		664	664	79	40	17	84	

202-04-27-27-27-10-02-27-11	LightHouse Point - Scenario B 2/18/2013		City:	ostal Code;	Naples	Naples		
Country: Co	lier County		Clien	Client Name:				
Analyst's Name:			Editi	on:	9th			
Land Use	Size	Daily - 2 W	ay Volume	PM Pea	k Hour	AM Pea	k Hour	
Land Ose	Size	Entry	Exit	Entry	Exit	Entry	Exit	
230 - Residential Condominium/Townhouse	80 m	265	265	33	17	7	36	
Reduction		0	0	O	0	0	0	
Internal		0	0	0	0	0	0	
Pass-by		0	0	0	0	0	0	
Non-pass-by		265	265	33	17	7	36	
210 - Single-Family Detached Housing	56 m	308	308	39	23	12	3.7	
Reduction		0	0	0	0	0	0	
Internal		Q	0	0	0	0	0	
Pass-by		0	0	O	0	ű	0	
Non-pass-by		308	306	39	23	12	37	
Total		573	573	7.2	40	19	73	
Total Reduction		0	0	0	0	0	0	
Total Internal		0	O O	Ó	0	0	0	
Total Pass-by		0	ņ	Ü	0	0	0	
Total Non-pass-by		573	573	7.2	40	19	73	

Note: Multi-Family Residential is described by ITE LU 230 as Residential Condominium/Townhouse.

Appendix C: Project Trip Distribution Map by Percentage (1 Sheet)



Appendix D: Scenario A – Turning Movements Detail (1 Sheet)

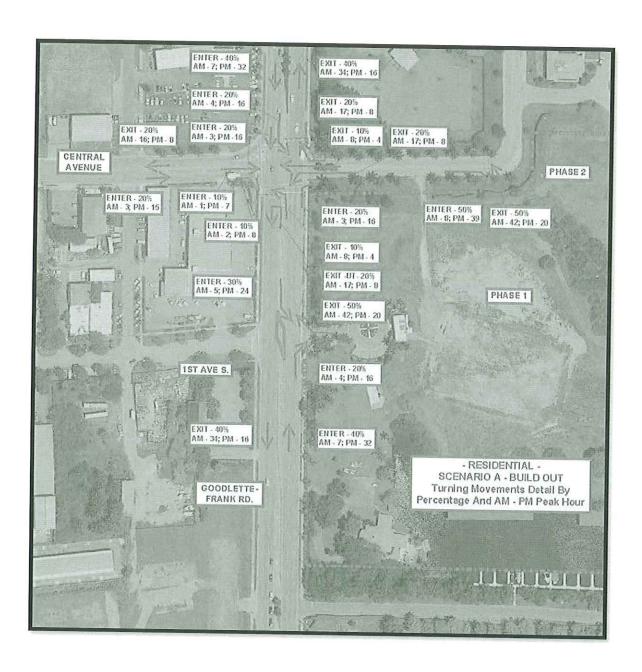


EXHIBIT D — LOS BUILD-OUT EVALUATION OF "D" DOWNTOWN

LOS BUILD-OUT EVALUATION OF 'D' DOWNTOWN

The following table has been prepared using traffic modeling information from the Collier County MPO's recent 2035 Long Range Transportation Plan (2035 LRTP) to indicate future conditions beyond build-out of the 'D' Downtown area. The time period selected was the year 2035, representing a 'post' build-out period (Note: The land-use data of the 2035 LRTP includes build-out development of the 'D' Downtown area. This analysis represents acceptable 'LOS' in the future years with the exception of the 8-lane bridge section of U.S.41; this road section, as all others, has no travel lane additions planned thru 2035. Future acceptable LOS on the 8 lane bridges will be subject of intermodal operations and signal system coordination.

ARTERIAL ROAD SEGMENT	LOS (1) CAPACITY	2012 (2) VOLUME	2035 (1) VOLUME
YI G. 44.			
U.S. 41:			
SR 90			
E. of CR851	70,900	65,711	87,309
W. of CR851	53,100	46,956	40,245
SR 45			
S. of 7 th AveN	53,100	42,593	38,959
S. of CR886	55,300	46,040	35,799
Goodlette-Frank Road			
S. of CR 886	55,300	32,618	47,302
Golden Gate Parkway			
W. of CR851	55,300	23,669	38,264
Central Avenue			
US41 to Goodlette	1,960 (3)	750 (3)	1,400 (4)

Notes: (1) AADT Data from Adopted LOS in 2035 LRTP.

- (2) AADT Data from Peak Season counts by City; count data is not factored.
- (3) Peak Hour data per City of Naples.
- (4) Factored ADT from Kimley-Horn 2025 Downtown Modeling.

DEC.	6,041	5,454	6,858	5,977
3RD QTR PEAK HOUR	466	494	437	460
SEPT.	4,771	4,683	4,331	4,284
2ND QTR PEAK HOUR	475	515	463	498
JUN.	4,799	4,849	4,855	2,000
1ST QTR PEAK HOUR	722	750	693	069
MAR.	7,359	7,040	066'9	6,717
ARTERIAL OR COLLECTOR STREET	CENTRAL AVENUE	CENTRAL AVENUE	CENTRAL AVENUE	CENTRAL AVENUE
TRAFFIC COUNT STATION NUMBER	62	62	62	62

Count Station 62 is located between US41 and 10th St N

		_	_	_
Year	2013	2012	2011	2010
SOT	m	В	М	В
LOS C VOL/CAP PEAK RATIO HOUR	0.37	0.38	0.35	0.35
LOS C PEAK HOUR	1,960	1,960	1,960	1,960
PEAK HOUR	722	750	693	069
4TH QTR MAXIMUM PEAK 2013 HOUR	7,359	7,040	066'9	6,717
4TH QTR PEAK HOUR	692	542	855	545

Two-way Volumes (Vehicles Per Day) For collector streets Arterials. In the City Of Naples

TRAFFIC	ARTERIAL	2008	2009	2010	Average	LOS
COUNT	OR	1 1			2008-2010	
NUMBER	COLLECTOR STREET	1 1				
	001 5511 6155 51616116					
8	GOLDEN GATE PKWY (CR 886)	22,484	21,159	22,903	22,182	С
10	GOODLETTE ROAD (CR 851)	34,103	33,199	28,970	32,091	С
15	US 41 (N OF CR 886)	49,598	46,182	47,461	47,747	C
16	US 41 (S OF CR 886)	47,912	44,447	45,880	46,080	C
19	US 41 (6 AV N/7 AV N)	43,944	41,082	42,430	42,485	C
23	US 41 (W OF CR 851)	46,837	44,200	44,277	45,105	С
24	US 41 (E OF CR 851)	68,762	64,939	65,088	66,263	С
30	PARKSHORE DRIVE	16,583	16,854	15,882	16,440	С
34	GULFSHORE BLVD N	6,121	5,792	5,762	5,892	В
37	HARBOUR DRIVE	5,845	5,551	5,920	5,772	В
38	CREECH ROAD	1,016	1,036	1,089	1,047	Α
39	MOORING LINE DRIVE	8,154	8,043	7,604	7,934	С
40	CRAYTON ROAD	9,366	8,610	8,576	8,851	С
43	22ND AVENUE NORTH	2,889	2,734	3,486	3,036	В
44	ORCHID DRIVE	4,095	4,023	4,336	4,151	В
45	FLEISCHMANN BLVD	5,541	5,976	6,465	5,994	С
48	GULFSHORE BLVD	6,862	6,940	7,157	6,986	В
49	BANYAN BLVD	2,988	6,561	2,489	4,013	Α
55	7TH AVENUE NORTH	5,420	4,994	4.800	5,071	С
56	10TH STREET	3,264	3,102	3,034	3,133	В
57	5TH AVENUE NORTH	3,784	3,856	3,839	3,826	В
62	CENTRAL AVENUE	7.049	6,903	6,717	6,890	В
63	8TH STREET	5.223	4,769	4,733	4,908	C
64	3RD AVENUE SOUTH	7,863	7,111	7,256	7,410	C
70	5TH AVENUE SOUTH	11,447	10.500	9,862	10,603	C
72	9TH STREET	8,251	8,156	8,957	8,455	C
76	BROAD AVENUE SOUTH	4,985	4,631	5,513	5,043	C
77	3RD STREET	6,061	5,529	5,378	5,656	В
79	GORDON DRIVE	9,037	8,052	7,726	8,272	C
83	SANDPIPER ST	6,852	6,232	6,320	6,468	C
100000	GULFSHORE BLVD SO	5,691	5,012	4,725	5,143	C
	4TH AVENUE NORTH	6,675	7,223	7,489	7,129	В
89	NEAPOLITAN WAY	7.797	6,674	7,751	7,407	В
91	WEST RD	5,080	4,695	4,734	4,836	В
	Totals	487,579	464,767	464,609	472,318	- 10